#### SOIL INVESTIGATION REPORT

FOR

CONSTRUCTION OF

OHSR

AT

### PH SECTION COMPOUND KAYAMKULAM

(Interim Report)



## CENTRE FOR INDUSTRIAL TRAINING CONSULTANCY & SPONSORED RESEARCH

COLLEGE OF ENGINEERING TRIVANDRUM

THIRUVANANTHAPURAM-695016

**DECEMBER-2022** 

# CENTRE FOR INDUSTRIAL TRAINING CONSULTANCY & SPONSORED RESEARCH

## COLLEGE OF ENGINEERING TRIVANDRUM

THIRUVANANTHAPURAM-695 016

# DEPARTMENT OF CIVIL ENGINEERING SUBSOIL INVESTIGATION REPORT

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Name of Client	: PROJECT MANAGER, PROJECT DIVISION, KWA, ALAPPUZHA
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Site of Investigation	: CONSTRUCTION OF OHSR AT PH
	SECTION COMPOUND, KAYAMKULAM
	: DECEMBER 2022
Job No. : CET/ITC&	SR No. 765/22-23
	••••••
Report No.	: (interim report)
Investigation done by	: Dr. JAYA V.
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- 11	

Principal Investigator

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ANNEXURE I

ANNEXURE II

#### 1. INTRODUCTION

The Project Manager, Project Division, KWA, Alappuzha requested the College of Engineering Trivandrum to conduct a subsoil investigation for the proposed OHSR at PH Section, Kayamkulam. Accordingly, the investigation was carried out under the guidance of Dr. Jaya V., Professor in Civil Engineering, College of Engineering Trivandrum, Thiruvananthapuram-16. The objectives of the work are to study in detail soil profiles and characteristics of samples collected from two borehole locations to obtain appropriate design parameters for safe and economical foundation design. The details of the subsoil investigation are given below:

#### 2. FIELD INVESTIGATION

Subsoil investigation was conducted using rotary boring rigs. After the site visit, it was decided to have two boreholes at the proposed site for OHSR at PH Section Kayamkulam. The two borehole locations are shown in Annexure I. The procedure of field investigation is as follows;

- The borehole is advanced to the depth at which the standard penetration depth has to be performed.
- Standard Penetration Tests were conducted at regular interval in three boreholes as per IS: 2131 – 1981.
- c. The bottom of the borehole is cleaned. The split spoon sampler attached to standard drill rods of required length is lowered into the borehole and rested at the bottom.
- d. The split spoon sampler is scated 150 mm by blows of a drop hammer of 63.5 kg falling vertically from a height of 750 mm. Thereafter, the split spoon sampler was further driven 300 mm in two steps each of 150 mm. The number of blows required to effect each 150 mm penetration was recorded. The first 150 mm of drive is considered as the seating drive. The total blows required for the second and third 150 mm of penetration is termed the penetration resistance N.

If the split spoon sampler is driven less than 450 mm (total), then N-value was taken as the number of blows for the last 300 mm penetration. In case, the total penetration is less than 300 mm for 50 blows, it was entered as rebound in the borelog.

- e. The split spoon sampler was then withdrawn and is detached from the drill rods. The split barrel is disconnected from the cutting shoe and the coupling. The soil sample collected inside the barrel is collected carefully and preserved for transporting the same to the laboratory for further tests.
- Standard penetration tests were conducted at every change in stratum or intervals of not more than 1.50 m whichever was less.

The subsoil investigation was conducted from 08/12/2022 to 14/12/2022. The Bore hole locations are shown in Annexure I. The soil profiles along with standard penetration test results for two bore holes are presented in bore log charts as Annexure II.

Representative samples using split spoon sampler were collected at regular intervals and preserved for conducting various identification tests in the laboratory. Water table in the borehole was carefully observed as per IS: 6935-1989. Table 1 shows the details of borehole numbers, depth of water table and maximum depth of boring drilling.

Table 1 Depth of Boreholes and Water Table Levels

Borehole No.	Maximum depth of borehole (m)	Depth of water table below EGL (m)
BH1	50.00	
BH2	50.00	

#### 3. LABORATORY STUDIES

The soil samples collected at every 1.5 m intervals from all boreholes were tested in the laboratory for classification. Depending on the type of substrata encountered, appropriate laboratory tests were conducted on soil samples.

#### 4. DESCRIPTION OF SOIL PROFILE AND BORELOG DETAILS

#### 4.1 Borehole: BH1 -Rotary Drilling

Top layer of 2.20 m is clayer sand. Layer of fine sand is noted until a depth of 7.20 m and followed by clay with shell dust till 13.40 m. The next layer is lateritic clay with sand extends to 20.30 m. Stiff clay extends until 37.20 m followed by fine sand to depth of 50.00 m. The borehole terminated at 50.00 m in fine sand.

#### 4.2 Borehole: BH2 -Rotary Drilling

Top layer of 2.60 m is clayer sand. Layer of fine sand is noted until a depth of 7.30 m and followed by clay with shell dust till 13.10 m. The next layer is lateritic clay with sand extends to 20.60 m. Stiff clay extends until 32.70 m followed by fine sand to depth of 41.30 m. The borehole terminated at 50.00 m in fine to medium sand.

#### 5. DESIGN CONSIDERATIONS AND RECOMMENDATIONS

Cast in situ Concrete Pile is recommended for the proposed Water Tank by considering the structural load and soil profile details observed in two boreholes. The recommended pile depth and carrying capacity of the piles are shown in Table 2 below. All the salient provisions and specifications of IS: 2911-2010 (Code of practice for design and construction of pile foundation) shall be closely adhered to.

The pile capacities recommended in Table 2 should be validated through a full-scale pile load test as per IS code provisions. The structural capacity of the piles shall be adequate. If the pile tips are terminated before the recommended depths the capacities will be lower and a chance of exceeding permissible settlement of foundation will be there. A minimum of two piles should be provided in a group.

Proper monitoring of the following points is essential during the construction of piles. The density of drilling mud shall be maintained as per standards and regular monitoring is required during construction. The cleaning of boreholes shall be ensured before the concreting of the pile. Lining may be provided during drilling if required due to the collapse of side soil. Close monitoring and documentation of the consumption of aggregates, reinforcement and cement shall be made. Details of chisei energy, depths achieved and time rate of progress shall be monitored and documented.

Pile termination at hard stratum shall be judiciously decided based on the acquired data during piling operations in conjunction with the details contained in this report

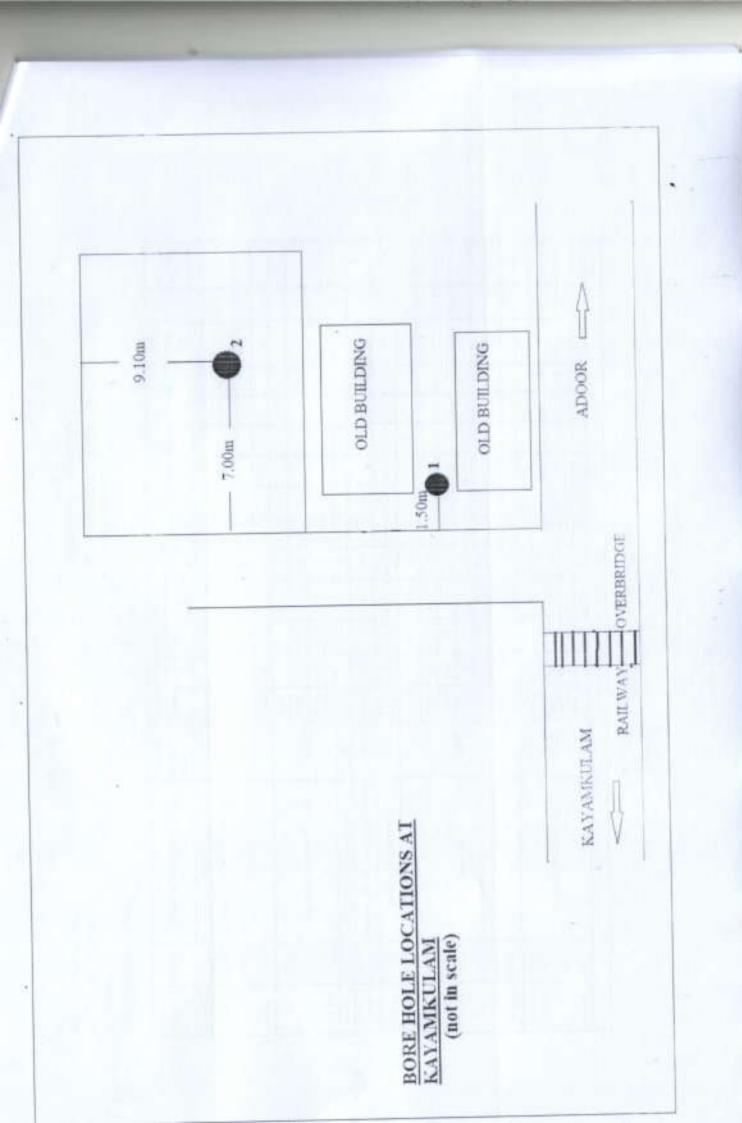
Table 2 Recommended Pile Capacities as per IS:2911-2010

	Borehole No	Depth of Pile (m)	Diameter of Pile (m)	Vertical Capacity (t)
		12.00	0.60	70.00
		42.00	0.75	82.00
-	BH1&BH2		0.80	94.00
			0.90	105.00
			1.00	115.00

General Notes: (i) Recommendations made are specific for the proposed site and imposed loads. (2) The Recommendations are based on the site investigation in two boreholes at the proposed water tank location. All depths shown in the bore log are from the existing ground level at the time of exploration. The existing ground level shall be recorded at a permanent datum using digital surveying before the start of work/excavations at the site.

(3) If any variation in the soil profile is met with during execution, fresh recommendations may be obtained from the Consultants or any qualified and experienced Geotechnical Engineer.

END OF REPORT



Project Proposed OHSR Sine Kayamkulam Bore Hole No.

Type of Boring, Botary Delling

Date of commence 08-12-3022 Date of empletion:- 10-12-2022 Gritand Water Levell-

			Theremose	Standard Penetiabili, 14 - 1700	PURCH			I	Combical properties that of N-value	e conton	-	_	often chile
	Pvefile	Visual Description of seel	of layers	Deptil	15	98	45	Value	Nagarata special speci	-	3	-	
3L(m)			(con)	CMD				4	0-10-10-20 20-30 30-40-46-50 >	>30	+	+	I
31,000			1	1 50		-		1-		1	+	+	
236		Charge saind	2.20	1.20			9.0	10	_		+	+	
				3.00.		-		-	1				
		Free const	5.00	4.50	ire.	4	+	00		-	t	+	
		A LINE DISTRICT		00.9	**	9	*	14		+	t	t	
7.30				7.50	-	-	-	44		+	+	+	
			200	90'6	+*	-	++	1.3		+	+	t	
		Clay with sand and theil dust	0.20	10.50	-	e4	-	1		+	+	t	
44.40				12.00	П					-		H	
14 60		Loteritic clay with sand	1.40	13.50	-	7	1	,	1	-	-	H	
		t accepts often with some and publics	1.80	15.00	œ	10	12	13	1	+	+	t	
16.60		The state of the s	1	10.00	1	-	9	11	\	+	+	t	
1	THE STREET	Experitic clay with sand	3.70	19.00	0	8	14	53		+	t	t	
26.30				21.60		51	10	28		+	+	t	
				23.00	6	=	1.0	33		+	+	1	
	H			25.00	10	133	113	30		+	t	T	
		Sufficial	16,00		10	7	17	32		-	t	t	
				30.00	+		11	20	\	+	t	T	
				11.00	8	01	13	23		+	t	T	
				36.00	L	0			7	+	t	T	
37.20			-	19.00	L	H	27	>50	4	1	1		
			_	00 17		H	H		Balmer-8 cm	+	T	Ī	
		The state of the s	03.80			37	13	99	Balance-11 cm.	+	t	T	
		Partie Suitte Styles Styles Little Artist				-			Balmoe-4 cm	+	t	T	
				10.00	1	H	100	-40 -40	Balance-19 cm	-	1	1	

Project : Proposed OHSR Site : Kayatokulam Bure Hole No.

Date of commence 12-12-2022 Date of completions 14-12-2022 Ground Water Levels-

Profess: Second Theoretical and shell dark Sand Theoretical and shell dark Sand Sand Sand Sand Sand Sand Sand Sand	1				Standard Functioning Test Data	Sunmit	there Te	st Det			-	Promo	city and
Clayer sind (1.20) 1500 2 2 2 2 4 4 6.10 10-20 20-30 30-40 40-50 30 -40 40-50 30 -40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 40-50 30 -40 40 -4	i i	Paulic	Visual Description of soil	Thickness of layers (m)	Daysh (m)	52	2	10	Vulne		-		(10) (%)
Claye's similary   2,60   1,50   2   2   2   3   4     Fine sand and shell dust   5,80   1,00   1   1   1   2   3     Laterptic clay with sand and shell dust   5,80   1,00   1   1   1   2   3   4     Laterptic clay with sand and shell dust   3,50   1,00   1   1   2   3   4     Laterptic clay with sand and   3,50   1,50   1,50   8   1,1   10   2,1     Laterptic clay with sand and   3,50   1,50   8   1,1   10   2,1     Laterptic clay with sand and   3,50   1,50   8   1,1   10   2,1     Laterptic clay with sand   3,50   1,50   8   1,1   10   2,1     Laterptic clay with sand   3,50   1,50   8   1,1   1,1   2,1     Laterptic clay with sand   3,50   1,50   8   1,1   1,1   2,1     Laterptic clay with sand   3,50   1,50   8   1,1   1,1   2,1     Shiff clay   1,2   0   1,5   1,8   1,1   2,1     Shiff clay   1,2   0   2,50   1,1   1,5   2,5     Halance Franciscum sand   8,70   1,8   1,4   1,6   5,5   1,4   1,6     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   5,5   1,4   1,4     Laterptic clay with sand   8,70   1,8   1,4   1,5   1,5   1,5     Laterptic clay with sand   8,70   1,8   1,4   1,5   1,5   1,5     Laterptic clay with sand   8,70   1,4   1,5   1,5   1,5   1,5   1,5     Laterptic clay with sand   8,70   1,4   1,5   1,5   1,5   1,5   1,5   1,5     Laterptic clay with sand   8,70   1,5   1,5   1,5   1,5   1,5   1,5   1,5     Laterptic clay with sand   1,5   1	90%									30-40 40-50		1	
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Fine sand and whell dust   5.80   6.00   7   9   10   19   19   10   19   10   10	9		Clayey Mind	4000	3.00	-	93	-	32			Ħ	
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Clay with sand and whell dust   2.80   10.50   1   2   2   4			4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		90.6	-	-	-	61			1	
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	90				59.00	E	7.	H	>50	Halanco-12 cm			